

George A. Austin P.E., L.S.  
d.b.a. Aqueous Fortis Consulting

Examination of Model data and reports  
on the City of Wichita, *Equus* Beds  
Aquifer and Storage Recovery Project

# Examination of Model data and reports on the City of Wichita, *Equus* Beds Aquifer and Storage Recovery Project

## Background.

The matter was brought to my attention via a phone call from Tessa Wendling, Attorney-at-Law, on November 1, 2018. Subsequent meeting with Ms. Wendling on November 13, 2018, highlighted her client's concerns and the direction of my examination of the materials available. The main purpose of my examination would be to review any aspects of the input and output data of the Models used to simulate the effects of the groundwater pumping and recharge elements and account for the City of Wichita's administration of the *Equus* Beds Aquifer Storage and Recovery Project (ASR).

## Procedure

Several publications were acquired through online means or were provided by Ms. Wendling. The publications were examined in connection to various concerns expressed by Ms. Wendling. Certain excerpts from the various publications were excerpted regarding the various concerns.

## Model Data Files

Several data files were downloaded. Examination of the various files provided some information as to input and output of the USGS MODFLOW 2000 Model. Originally It was believed comparison the data sets used in different MODEL runs would be of value. After review of the data sets, it was determined that further comparisons would be better served based on the reported results in the various report.

## Review of Reports

[U.S. Geological Survey Scientific Investigation Report 2013-5042](#), Simulation of Groundwater Flow, Effects of Artificial Recharge, and Storage Volume Changes in the *Equus* Beds Aquifer near the City of Wichita, Kansas Well Field, 1935–2008, reports on the MODFLOW model used to simulate the effects of groundwater pumping, artificial recharge, precipitation, river and stream interactions, natural recharge and other factors on groundwater. Operation of MODFLOW, as calibrated, assigns quantifiable volumes for each of these effects.

There are scale and time distributions that limits the Model. “2. The groundwater-flow model was discretized using a grid with cells measuring 400 ft by 400 ft. Model results were evaluated on a relatively large scale and cannot be used for detailed analyses such as simulating water level drawdown near a single well. A grid with smaller cells would be needed for such detailed analysis.” (p. 72, Model Limitations) and “Although irrigation pumpage was assumed to occur only in May through August, annual irrigation rates were calculated and used in the simulation.” (p. 43). Additionally, “Groundwater pumpage data for 1935 to 1979 were obtained from Spinazola and others (1985) and Myers and others (1996). Groundwater pumpage for the stress periods from 1935 through 1979 was distributed in the model based on the spatial and temporal distribution of pumping in Spinazola and others (1985). The model cells from Spinazola and others (1985) are 1 mile on each side and pumping was assigned to the center of each cell. Pumping wells were placed in the current model to coincide with the center of each cell in the model from Spinazola [highlight added] and others (1985). Pumping was distributed vertically across

all model layers by using the MultiNode Well Package. Locations of simulated pumping wells for 1935 through 1979 are shown in figure 29.” (p.39, Wells)

The grid scale and using annual rates rather than a distributed rate based on usage and the one-mile square grids are important because it shows the large scale of the impacts. The “cone of depression” of each well is aggregated at the one-square mile grid for 1935-1979 (rather than the 400’ square grid for pumping after 1980 and other elements) which creates a deeper depression at one location rather than at the point of the actual well diversion. Clearly the MODFLOW Model does not address individual well impacts either in scale or location.

One of the priorities of the ASR was regarding the saltwater intrusion from the Burton well field and the Arkansas River. *“In March 2006, the city of Wichita began construction of the Equus Beds Aquifer Storage and Recovery project to store and later recover groundwater, and to form a hydraulic barrier to the known chloride-brine plume near Burrton, Kansas. In October 2009, the U.S. Geological Survey, in cooperation with the city of Wichita, began a study to determine groundwater flow in the area of the Wichita well field, and chloride transport from the Arkansas River and Burrton oilfield to the Wichita well field.”* (p.1, Abstract) And *“Sophocleous (1983) simulated chloride transport in the Equus Beds aquifer, Spinazola and others (1985) developed a model to simulate groundwater flow and chloride transport in the Equus Beds aquifer and underlying Wellington Formation...”* (p.6, Previous studies)

### [US Geological Survey, Scientific Investigations Report 2010–5023. Water Quality in the Equus Beds Aquifer and the Little Arkansas River Before Implementation of Large-Scale Artificial Recharge, South-Central Kansas, 1995–2005](#)

The Study of the movement of the chloride in the *Equus Beds* as reported by SIR 2010-5023 provides the basis of the ASR water quality. “ *The primary sources of chloride to the Equus Beds aquifer are from past oil and gas activities near Burrton and from the Arkansas River. Computed chloride concentrations in the Little Arkansas River near Halstead exceeded the Federal SDWR of 250 mg/L about 27 percent of the time (primarily during low-flow conditions). Chloride concentrations in groundwater exceeded 250 mg/L in about 8 percent or less of the study area, primarily near Burrton and along the Arkansas River. Chloride in groundwater near Burrton has migrated downgradient about 3 miles during the past 40 to 45 years. The downward and horizontal migration of the chloride is controlled by the hydraulic gradient in the aquifer, dispersion of chloride, and discontinuous clay layers that can inhibit further downward migration. Chloride in the shallow parts of the Equus Beds aquifer migrated less than 0.5 mile during the past decade. Migration is slower because of the decrease in the hydraulic gradient since 1992. On the basis of these results, artificial recharge (especially at depths of 100 to 150 feet) could create an effective barrier to saltwater migration.*”(p. 1, Abstract) The rise of groundwater elevations in the Basin Storage Area would lessen the groundwater hydraulic gradient and therefore, movement of the chloride would be slowed. Of course, the barrier to saltwater migration is related to the Wichita well field. There’s no forecast as to whether the chloride plume will move in a different direction nor if that movement would be accelerated. Generally, comparison of groundwater elevations does not indicate a change in direction.

### [ASR Permit Modification Proposal Revised Minimum Index Levels & Aquifer Maintenance Credits by Burns & McDonnell, 3/21/2018.](#)

The studies also do not address the lowering of the index elevation the 1993 levels which were historical lows. The pumping to the lower levels would increase hydraulic gradients and potentially accelerate the movement of chlorides.

Another important element in the Modification Proposal is changing the recharge accounting from the current Model driven accounting to a routine calculation. “A one-time, five percent (5%) initial loss will be deducted from the total number of AMCs applied in each index cell. This initial loss accounts for losses to the aquifer inherent in the injection and recovery process. An average annual recurring loss of three percent (3%) will be applied annually to recharge credits to account for recharge credit migration from the BSA. This recurring loss will be gradational geographically across the BSA...” (p. 4-3) In the model-based accounting, based upon the report’s tabulation, the increase in the accumulated credits from 2006 to 2015 would increase from 4978 acre-ft to 5867 acre-ft (p. 4-6, Table 4-2). The values used are the calculated recharge credits from Model calculations. The Aquifer Maintenance Credits would be based on the City of Wichita diversions from the Little Arkansas River, which are reported in the Recharge credit reports. The amount in 2015 was reported at 1,048 acre-ft of the 2925 af diverted or about 36%. (p.2-4, Aquifer Storage and Recovery Project,2015 Annual Accounting Report prepared for City of Wichita, Kansas February 2018) Approximately, 9 percent of the water pumped to the water treatment plant was used in operation. This would indicate if AMC concept is adopted the initial losses should be at least that 9%.

The authors of this report had available to them, recharge values based on river flows infiltrating in to the Basin Storage Area Index Cells or outflowing to the river. In the USGS SIR 2013-5042, the dilemma of assigning a credit to use of river water rather than groundwater pumping results in the following contradiction “Increasing recharge either increases flow from the aquifer to the Arkansas and Little Arkansas Rivers or decreases flow from the rivers to the aquifer.” (p. 66 *ibid*). The previous quote offers two choices in the impact of increasing recharge, when really the it is a composite of the two. For instance, in the same report, during the steady-state simulation flow budget, inflow into the groundwater system represents 30.5% of the total inflows second only to the 64.7% from natural recharge but represents 51.8 % of the total outflows. (See Reproduced Table 8 below.) (p.48 *ibid*.)

Table 8. Steady-state calibration simulation flow budget.  
[ft<sup>3</sup>/day, cubic feet per day; acre-ft/day, acre feet per day; --, not applicable]

Budget component	Flow rate, in ft <sup>3</sup> /day	Flow rate, in acre-ft/day	Percent of total flow
Inflow			
Head dependent boundaries	2,320,409	53.3	4.7
Recharge	31,855,858	731.3	64.7
River leakage	15,024,649	344.9	30.5
Well pumping	0	0.0	0.0
Total in	49,200,916	1,129.5	100
Outflow			
Head dependent boundaries	1,167,715	26.8	0.2
Evapotranspiration	18,569,682	426.3	38.8
Drains	2,129,863	48.9	4.6
River leakage	25,165,966	577.7	51.8
Well pumping	2,204,735	50.6	4.6
Total out	49,237,960	1,130.3	100
Total in - out	37,044	0.9	
-Percent difference	-0.08	-0.08	

In 2015, the Aquifer Storage and Recovery Project, 2015 Annual Accounting Report prepared for City of Wichita, Kansas February 2018, reported that *“Infiltration from the Little Arkansas River throughout the Basin Storage Area was approximately 5204 acre-feet, ... The model shows that a total of 38,717 acre-feet of water migrated from the aquifer in the Basin Storage Area to the Little Arkansas River in 2015.”* (P. 4-7.)

## Conclusion and Findings

The review of the data and the reports indicate substantial scientific fit to the measured conditions. However, the MODFLOW model cannot be used to look at individual impacts with any degree of certainty. The scale both geographically and temporally are large enough that impacts are general to the study area rather than specific to any one location.

The Aquifer Maintenance Credit concept doesn't take in account the fact the stream flow diverted to the water treatment plant and then piped to Wichita is in part, outflow from the aquifer and the diversion stream flow is subtracting from water available for aquifer infiltration. In 2015, the infiltration from the Little Arkansas River was 5,204 AF and outflows to the Little Arkansas River were 38,717 AF. This would indicate that if a proportional factor based on the infiltration to total surface-water/groundwater interaction were applied that only 11.8 percent of the diversion would possibly be assigned as AMC. At the steady-state, the proportion of the total surface water – groundwater interaction, is 38 percent infiltration. This indicates that the quote from the MODFLOW report: *“Increasing recharge either increases flow from the aquifer to the Arkansas and Little Arkansas Rivers or decreases flow from the rivers to the aquifer”* reflect the rise in recharge storage levels.

Water quality related to the chloride plume indicated that the restoration of the Equus Bed Aquifer to historic levels does serve as a barrier to movement towards the Wichita well field. The studies do not forecast future movement, though pumping the aquifer to levels below historical levels would certainly accelerate movement towards the pumping source.

The references and their hyperlinks that I reviewed at least partially are listed below

<https://pubs.usgs.gov/sir/2010/5023/> Water Quality in the Equus Beds Aquifer and the Little Arkansas River Before Implementation of Large-Scale Artificial Recharge, South-Central Kansas, 1995–2005 Scientific Investigations Report 2010–5023

<https://pubs.usgs.gov/sir/2013/5042/sir2013-5042.pdf> Simulation of Groundwater Flow, Effects of Artificial Recharge, and Storage Volume Changes in the Equus Beds Aquifer near the City of Wichita, Kansas Well Field, 1935–2008. Scientific Investigations Report 2013–5042.

<https://pubs.usgs.gov/sir/2014/5185/pdf/sir2014-5185.pdf> Status of Groundwater Levels and Storage Volume in the Equus Beds Aquifer near Wichita, Kansas, 2012 to 2014. Scientific Investigations Report 2014–5185.

<https://pubs.usgs.gov/sir/2016/5042/sir20165042.pdf> Effects of Aquifer Storage and Recovery Activities on Water Quality in the Little Arkansas River and Equus Beds Aquifer, South-Central Kansas, 2011–14 U.S. Department of the Interior U.S. Geological Survey Scientific Investigations Report 2016–5042

<https://pubs.usgs.gov/sir/2015/5121/sir20155121.pdf> Groundwater-Level and Storage-Volume Changes in the Equus Beds Aquifer near Wichita, Kansas, Predevelopment through January 2015 Scientific Investigations Report 2015–5121

<https://pubs.usgs.gov/sir/2016/5165/sir20165165.pdf> Status of Groundwater Levels and Storage Volume in the Equus Beds Aquifer near Wichita, Kansas, January 2016 Scientific Investigations Report 2016–5165

<https://www.agriculture.ks.gov/WichitaASR> Division of Water Resources, Kansas Department of Agriculture. Link to various reports and documents.

INVESTIGATION REPORT  
ON USE OF SURFACE WATER FOR  
ALTERNATIVE MAINTENANCE CREDITS

By

Carl E Nuzman, P.E., P.Hg.  
Consulting Engineer/Hydrogeologist

33314 NW Huxman Rd  
Silver Lake, KS 66539

Cell 785 224 9929  
[cenuzman@gmail.com](mailto:cenuzman@gmail.com)

For

Water Appropriators of GMD 2

November 28, 2018

## INTRODUCTION

The City of Wichita, Kansas has developed an artificial ground water recharge system to supplement the natural recharge from precipitation to the Equus Beds aquifer from which the City has depended on for a major portion of the city water supply for many years. In spite of some short-comings, artificial recharge to the aquifer has been achieved.

The source of recharge water has been flows in excess of Minimum Desirable Streamflows from the Little Arkansas River (K.S.A. 82a-703c) and from infiltration of river water by means of wells adjacent to the river. During extended drought conditions, artificial recharged is needed for the City to use its allocated appropriation quantity of water established under senior water rights. As precipitation returns to more normal annual precipitation and streamflow's are available, the need for artificial recharge becomes limited.

The City is now asking the Chief Engineer of the Division of Water Resources, KDA, to approve special groundwater recharge credits, now called Alternative Maintenance Credits (AMC) to the Equus Beds Aquifer for the direct use of surface water from Little Arkansas River directly to a water treatment plant for public consumption. This action if approved would have the effect of illegally increasing the available appropriation of water under existing senior water rights of the City of Wichita to the potential detriment of other appropriators from the same local source of supply.

## BACKGROUND

In other states, their water laws distinguishes between surface water, that water drawn directly from a stream or river for beneficial use, largely for irrigation or mining, the first in Time is the first in Right. Ground water use came along later and followed somewhat the same laws as was established for surface water. However several variations exist among the several states. This doesn't work well with wells of junior water rights located between or near wells with senior rights since the direct mutual interference between wells is limited. Wells need to be considered as individual straws all pulling from the same common source of supply.

Kansas Water Appropriation Act made no specific distinction between surface and ground water rights. This was good in some ways and poor in other ways. Traditionally, the direct use of surface water is not mixed with ground water rights and the water rights are kept separate. It has been well established that the pumpage of ground water from wells where the aquifer is connected to the river system can greatly diminish stream flow. Likewise consumptive diversion of stream flow limits the availability of surface water to recharge the aquifer where they are connected.

The Equus Beds aquifer has several sources of recharge; the Little Arkansas River on the east, natural recharge from precipitation and return flows from irrigation spread over the area, some inflow from the Arkansas River to the southwest, some subsurface inflow from the north, and

some leakage from the surrounding bedrock formations. This aquifer has never had aquifer storage depleted to the point that it would never recover like the Ogallala aquifer to the west.

## EVALUATION TOOL

To evaluate and make aquifer calculations we have a wonderful tool called MODFLOW developed by staff members of the USGS that uses a finite-difference computing technique. This model is well adapted to our present day digital computing systems but has some limitations. The model is excellent in calculating the volume of change from one general water level to another assuming there is a uniform storage coefficient and specific yield that applies equally, everywhere in the model.

We assume the hydraulic conductivity is constant throughout each layer of the model, but since this is an alluvial deposit, the depositing stream flow energy varied greatly from place to place throughout the area changing the hydraulic conductivity. That's always evident in locating wells in that usually three or more test holes have to be drilled to find the best location for a high capacity well site.

MODFLOW has difficulty simulating the five (5) sources of recharge inflow to the aquifer because these are highly variable and inflow rates can vary with the head differential and areal location. The inflow from the Little Arkansas River not only varies with head differential but with wetted perimeter, and variations in riverbed conditions that are associated with changes in geology.

Infiltration from precipitation varies with soil type, field management technique used, soil cover or lack thereof, type and intensity of precipitation and antecedent conditions at time of precipitation occurrence.

The Arkansas River to the southwest has over the years developed a water quality issue that needs to be recognized in any evaluation. Fortunately due to the distance and low hydraulic gradient in that direction it is not of great concern at this time.

The effect of inflow from the north can be minimized by extending the model area in that direction.

The leakage from the surrounding bedrock and possible leakage from the bottom is unknown and thought to be within the margin of error of the calculations.

The MODFLOW model is a good tool but not perfect by any means. The primary deficiency is in simulating real world boundary inflow from the original steady state conditions, for changed conditions, and how stream infiltration and boundary inflow can change spatially.

## THE ISSUE

At issue is, “Can the direct use of surface water to a water treatment plant” be used for groundwater recharge credits, renamed water maintenance credits, when the water never ever entered the aquifer?

A review of the Kansas Water Appropriation Act in K.S.A. 82a-701, Definitions does not mention surface water or ground water. K.S.A. 82a-703a, 703b, and 703c list minimum streamflows. Streamflow implies movement of the visible portion of water moving down gradient in a natural channel. K.S.A. 821-707(d) contains the sentence; *Any surface water right held in the custodial care of the state shall neither directly benefit nor impair any other surface water right within the stream reach designated for recovery.* Surface water diversion relates to that portion of water removed from a flowing stream or river to which the owner has developed a water property right to use in accordance with the governing law.

K.S.A. 82a-711(b) In ascertaining whether a proposed use will prejudicially and unreasonably affect the public interest, the chief engineer shall take into consideration: (1) Establish minimum desirable streamflow requirements; (2) the area, safe yield and recharge rate of the appropriate water supply; {At the time this Act was passed this referred only to the natural recharge to the aquifer} (3) ...;(4) ...; (5) all other matters pertaining to such question.

If the use of water maintenance credits does in fact constitute an additional appropriation from the Equus Beds aquifer then such action should be acted on as a new appropriation.

In K.S.A. 82a-711a., it shall be an express condition of each appropriation of surface or ground water that the right of the appropriator..... This sentence implies that surface water rights and ground water right are two separate rights.

In accordance with historic use and practice, water rights to the diversion of surface water and water rights to the use of water extracted from the ground by means of wells should not be intermixed.

K.S.A. 82a-706a provides the authority for the chief engineer to promulgate and enforce such rules, regulations and standards necessary to achieve the purpose of this act.

In K.A.R. 5-1-1 Definitions (g), “Artificial recharge” means the use of source water to artificially replenish the water supply in an aquifer.

The source water certainly can be streamflow from the Little Arkansas River but it clearly states the use is to artificially replenish the water supply in an aquifer not to divert the water by pipeline directly to a water treatment facility. Further the City of Wichita is asking for water recharge credits to withdraw ground water at a later date which then would constitute an additional appropriation of ground water in excess of existing senior water rights.

K.A.R. Article 5-12 specifically covers aquifer storage and recovery and requires appropriate accounting of the recharge of water to the aquifer and the withdrawal of said water.

### ALTERNATE USE

It is suggested as an alternate use that some of the streamflow in excess of minimum desirable stream flows from the Little Arkansas River be used to maintain a hydraulic barrier to prevent the Burrton area contamination plum from spreading further into the Equus Beds aquifer.

Approximately 85% or more of the injected water in the hydraulic barrier can be recovered down gradient, which water can then be removed by wells and used directly by the city. This would be a true maintenance use of the water.

### IN SUMMARY

The direct use of surface water to a water treatment plant to establish aquifer maintenance credits for future aquifer use is in direct conflict with the definition of aquifer recharge listed in K.A.R.5-1-1(g).

The use of the aquifer model MODFLOW to establish aquifer maintenance credits seems somewhat like a smoke screen to provide ground water recharge credits that are not consistent with present Rules and Regulations and are avoiding the main purpose of the aquifer storage and recovery project. When the aquifer is full its full and developing additional ground water recharge credits from surface water use is inconsistent with the intent of the water laws.

STATE OF KANSAS  
BEFORE THE DIVISION OF WATER RESOURCES  
KANSAS DEPARTMENT OF AGRICULTURE

In the Matter of the City of Wichita's )  
Phase II Aquifer Storage and Recovery Project )  
in Harvey and Sedgwick Counties, Kansas )  
\_\_\_\_\_ )

Case No. 18 WATER 14014

Pursuant to K.S.A. 82a-1901 and K.A.R. 5-14-3a.

**TECHNICAL ASSESSMENT**

**ASR PERMIT MODIFICATION PROPOSAL  
REVISED MINIMUM INDEX LEVELS & AQUIFER MAINTENANCE CREDITS**

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**CONTENTS**

Introduction..... 1  
Hydrologic effects of 1% drought simulation in proposal..... 3  
    Hydrologic Budget Analysis ..... 5  
        Scenario A – City Pumping 40,000 AF/y (without ASR Credit)..... 5  
        Scenario B – City Pumping ASR Recharge Credit to Current Minimum Index Level ..... 5  
        Scenario C – City Pumping ASR Recharge Credit to Proposed Minimum Index Level..... 5  
    Discussion of Budget Analysis ..... 6  
Drawdown to Aquifer Water Levels and Well Impacts..... 7  
Water Quality Change ..... 9  
Proposed ASR Accounting Methodology ..... 10  
Summary and Conclusions ..... 10  
Qualifications and Experience ..... 13  
References ..... 15  
Figures - 1 through 8 (end of report)

**INTRODUCTION**

Balleau Groundwater, Inc. (BGW) has been retained by Wendling Law, LLC and Adrian & Pankratz, P.A. to provide technical information and opinions regarding hydrologic effects associated with City of Wichita ASR Permit Modification Proposal (hereafter “the Proposal”) (Burns & McDonnell, 2018a). Wendling Law, LLC provided model files and a transmittal (Macey, 2018)

25 with the files associated with the City of Wichita Aquifer Storage and Recovery (ASR) Permit  
26 Modification Proposal. The City's ASR Permit Modification Proposal includes analysis based on  
27 the USGS model (Kelly and others, 2013) of the Equus Beds aquifer. Burns & McDonnell (BMcD)  
28 performed the model analysis. The USGS model provides a technical basis for the City's Proposal  
29 and for annual accounting (Burns & McDonnell, 2018b) of ASR recharge credits compatible with a  
30 condition of approval in existing permits required to capture, store and recover water for the City's  
31 beneficial use. BGW's assessment of hydrologic effects related to the City's Proposal emphasizes  
32 information contained in the Proposal and considers related technical documents (listed in the  
33 references section of this report). BGW also obtained a copy of the original distribution of the  
34 USGS model to inspect the files and modeled water budget.

35

36 As part of the technical assessment, we inspected and ran the model files the City (Macey,  
37 2018) provided and compared the files to the original USGS files. In the course of doing so, we  
38 observed a difference between specifications in the USGS model and the model provided by the  
39 City, namely the ratio of horizontal to vertical hydraulic conductivity simulated between model  
40 layers. BMcD reports that no changes were made to the original construction or hydrogeologic  
41 properties of the original USGS model (Burns & McDonnell, 2018a, p. 2-7 and 2018b, p. 4-1). The  
42 reason for the change is not clear. The upshot is that we found the alteration to model specifications  
43 does not result in a significant change to certain technical aspects we evaluated in the Proposal. For  
44 example, we evaluated the BMcD analysis that defines the proposed minimum index levels in both  
45 model versions and found the change is on the order of a few feet or less. However, unless there is a  
46 reason to deviate from the original USGS model concept, we recommend the City accounting of  
47 recharge credit and the analysis in the Proposal be updated accordingly to confirm that other  
48 potentially significant factors do not turn up. Keeping the ratio of horizontal to vertical hydraulic  
49 conductivity the same as in the USGS model should improve simulation of hydrologic conditions in  
50 west Harvey County (northwest corner of the basin storage area) where anisotropy between model  
51 layers is known to occur. The description of the ratio of horizontal to vertical hydraulic conductivity  
52 represented in the model is described by the USGS (Kelly and others, 2013, p. 34).

53

54 In our technical assessment, we evaluated the City's Proposal using the model as provided  
55 by the City and using the USGS model in its original form. We found that the results in both sets of  
56 model simulations are not different enough to affect our overall conclusions. To remain consistent  
57 with the original USGS concept, herein we present results from our assessment with the model in its  
58 original USGS form.

59

60 **HYDROLOGIC EFFECTS OF 1% DROUGHT SIMULATION IN PROPOSAL**

61

62 BMcD describes the 1% drought simulation in the Proposal to stem from a decision by the  
63 City to utilize a 1% exceedance probability drought for resource planning of future water supplies.  
64 The decision prompted BMcD to develop a drought analysis with the USGS model to assess  
65 hydrologic conditions in the Basin Storage Area (BSA). In the process of evaluating scenarios of  
66 prolonged drought, BMcD found that some water levels in the BSA<sup>1</sup> are projected to drop below the  
67 minimum index water levels, which triggers a condition that prevents diversion of ASR recharge  
68 credit water in the City's current ASR permit. Accordingly, BMcD's analysis is the technical basis  
69 for the Proposal to revise the minimum index levels by lowering them so the City could divert ASR  
70 recharge credit water during an extended drought.

71

72 The BMcD analysis in the Proposal presents results of the 1% drought simulation in the  
73 context of water-level elevations and percent of saturated thickness of the aquifer. The results are  
74 based on the total City pumping (non-credit and ASR recharge credit) represented in the 1% drought  
75 simulation (Burns & McDonnell, 2018a, Table 2-5). The model has the capability of isolating  
76 hydrologic effects from components of City pumping. For example, the BMcD 1% drought analysis  
77 can be adapted to quantify the hydrologic effect of pumping the ASR recharge credit. **Figure 1**  
78 shows how the hydrologic system responds to City ASR recharge credit pumping in the 1% drought  
79 simulation. Initially, the pumping produces most of the water from aquifer storage, but as pumping  
80 continues, the cone of depression from groundwater pumping induces (depletes) flow from the Little  
81 Arkansas and Arkansas rivers. A notable observation on **Figure 1** is that stream depletion continues  
82 to occur for years after groundwater pumping ceases. This lagged depletion response occurs  
83 because, even though pumping has stopped, stream depletion continues to fill in the cone of  
84 depression that was caused when the well was pumping.

85

86 The proximity of the City wells to the rivers results in groundwater operations  
87 (diversion/injection) affecting river flow within one year of pumping. Below we expand on this  
88 technical approach of analyzing hydrologic effects from different components of City pumping  
89 (**Figure 1**) with an examination of hydrologic effects that considers an example of diverting  
90 groundwater that causes drawdown to the level of the proposed minimum index level.

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<sup>1</sup> In the Proposal, BMcD reports that water levels in about half of the index cells lowered below the current minimum index level in their 1% drought analysis (Burns & McDonnell, 2018a, Table 2-10).

91

92           The BMcD 1% drought analysis results in some water levels in the BSA dropping below the  
93 current minimum index level, thereby preventing the City from diverting ASR credit water. To  
94 clarify, the revised minimum index levels in the Proposal do not directly represent the modeled  
95 water levels in the BMcD drought analysis. To determine the revised minimum index level in the  
96 Proposal, BMcD added a contingency to the water levels modeled at the end of the drought  
97 simulation. That is, the proposed minimum index levels are at a lower elevation than that modeled  
98 in the 1% drought analysis. We are interested in quantifying hydrologic effects associated with the  
99 City potentially diverting ASR recharge credit from the depth limited by the proposed minimum  
100 index level. Accordingly, to quantify the hydrologic effects associated with the Proposal, model  
101 analysis in addition to the BMcD scenario is needed.

102

103           In our assessment of City groundwater pumping, we used the model to quantify hydrologic  
104 effects from three categories of pumping: **A)** diversion of groundwater without ASR credit (i.e.  
105 pumping 40,000 AF/y), **B)** diversion of ASR recharge credit water with the constraint of the existing  
106 minimum index level (1993 level) and **C)** diversion of ASR recharge credit water with the lowering  
107 of the existing minimum index level to the proposed level. The analysis approach allows for  
108 quantifying the potential hydrologic effect of the Proposal (i.e. presenting an example of hydrologic  
109 effects if the City diverted groundwater that caused drawdown to the proposed minimum index  
110 level). An assessment of the categories of pumping associated with causing drawdown to the  
111 minimum index levels is possible with the Multi-Node Well package (Konikow and other, 2009) that  
112 is used in the USGS model.

113

114           The MNW package has utility for analyzing well yield that is limited in association with a  
115 lowered pumping water level, typically near the pump intake. For example, when the pumping  
116 water level in a well approaches the pump intake, a threshold is eventually crossed when the yield  
117 must decline to prevent air from entering the intake. That threshold can be set as a limit in the  
118 modeled representation of pumping wells. The same concept can be used to estimate credit water  
119 diverted from City wells by setting a limit that matches the minimum index water level. Running a  
120 series of simulations compatible with the BMcD 1% drought scenario, with limits set at both the  
121 current (1993) and proposed minimum index levels for ASR credit pumping, allows for analyzing  
122 the hydrologic effects of the three categories of pumping described above.

123

124 **Hydrologic Budget Analysis**

125

126 Scenario A – City Pumping 40,000 AF/y (without ASR Credit)

127

128 The pumping schedule to assess the effect of pumping without ASR credit is based on the  
129 City goal of using 40,000 acre feet per year (AF/y) from the Equus Beds wellfield (EBWF) prior to  
130 use of ASR recharge credits (Burns & McDonnell, 2018a, p. 2-5).<sup>2</sup> **Figure 2** shows the hydrologic  
131 system response to the City pumping 40,000 AF/y during the 1% drought scenario. In the first year  
132 of pumping, approximately 20 percent of the pumping amount (10 cfs) is depleted from the river  
133 system; by the second year, about 35 percent of the pumping amount (20 cfs) is depleted.

134

135 Scenario B – City Pumping ASR Recharge Credit to Current Minimum Index Level

136

137 The pumping schedule for diverting ASR recharge credit is based on using the model to  
138 solve for the amount of credit water that can be diverted from above the current minimum index  
139 level. The analysis is derived by running the 1% drought scenario with the City goal of diverting  
140 40,000 AF/y and using the MNW-well capability to determine the amount of water remaining  
141 above the current minimum index levels that can also be diverted. That amount of water remaining  
142 is the ASR recharge credit water that can potentially be diverted with the City's existing ASR permit  
143 (subject to also pumping 40,000 AF/y). The analysis allows for isolating the potential ASR recharge  
144 credit water that can be diverted in the 1% drought scenario. **Figure 3** shows the quantity of that  
145 water and the hydrologic system response of pumping it. The total amount water diverted is about  
146 14,900 acre feet, which indicates that, if the City prioritizes pumping 40,000 AFY to pumping ASR  
147 recharge credit, much of the water diverted from above the current minimum index level is to satisfy  
148 the goal of diverting 40,000 AF/y.

149

150 Scenario C – City Pumping ASR Recharge Credit to Proposed Minimum Index Level

151

152 Analyzing the effect of the Proposal (Scenario C) is similar to Scenario B, except the limit on  
153 the minimum index level in the MNW wells is lowered from the current permitted level to the  
154 proposed level. The analysis identifies the potential ASR recharge credit that could be diverted if

---

<sup>2</sup> This is similar to the BMcD 1% drought scenario which simulates approximately 40,000 AF/y of non-credit water diverted from the EBWF in all eight years of the drought, except the first year when 34,202 AF is diverted.

155 drawdown from City pumping occurred to the level of the proposed minimum index level. **Figure 4**  
156 shows the hydrologic system response to that diversion and indicates the amount of ASR recharge  
157 credit water that could produced is 79,500 acre feet, which is in addition to the 14,900 acre feet  
158 produced in Scenario B.

159

#### 160 Discussion of Budget Analysis

161

162 The hydrologic budget analysis provides insight to system response in the context of City  
163 pumping that causes drawdown to the proposed minimum index level. Points are apparent  
164 regarding stream depletion in consideration of minimum desirable streamflow (MDS) and surface-  
165 water availability.

166

#### 167 ***Minimum Desirable Streamflow***

168

169 The permit that regulates the City's ASR project restricts the recovery of recharge credits to  
170 periods when water levels are above the established minimum index level (Burns & McDonnell,  
171 2018a, p. 1-1). The proposal seeks to revise the minimum index level to a lower elevation, which  
172 would allow a new diversion of groundwater. **Figure 4** shows the potential credit water that could  
173 be diverted results in up to 10 cubic feet per second (cfs) of depletion to the Little Arkansas and  
174 Arkansas rivers

175

176 The minimum desirable streamflow (MDS) established at the gage on the Little Arkansas at  
177 Valley Center is 20 cfs.<sup>3</sup> **Figure 5** shows a chart of flow at that gage during the two years (2011 and  
178 2012) that characterize the 1% drought scenario. During that time, flow at the gage is below 20 cfs  
179 49 percent of the time. A change in flow of 5 cfs at that gage (assuming about half of the impact  
180 occurs on the Arkansas River) during the drought, would impact MDS flow so that it is less than 20  
181 cfs about 53 percent of the time (**Figure 5**). The percentages translate to about one month of MDS  
182 flow not met due to drawing down water levels from the current minimum index level to the  
183 proposed level.

184

185 The City wells are located in between the Little Arkansas and Arkansas rivers and analysis  
186 indicates that diversion/injection of water into the BSA affects river flow. During times when water

---

<sup>3</sup> The minimum desirable streamflow on the Little Arkansas River is 20 cfs every month of the year (K.S.A. 82a-703c.)

187 levels are low in the BSA, injection of credit water into the aquifer will initially fill aquifer storage  
188 and eventually add flow to the river system and to evapotranspiration. However, during times of  
189 drought, when MDS flow is generally of concern, the Proposal seeks to recover credit water from  
190 below the current minimum index level, which will cause a new depletion to the river system that  
191 impacts MDS flow.

192

### 193 ***Surface-water availability***

194

195 The USGS model simulates the Little Arkansas and Arkansas rivers as a boundary condition  
196 that does not account for total streamflow. That is, if segments of the river near the City dry out or  
197 have low flow during a drought, the model does not account for it. In that setting, there is potential  
198 for the model to overestimate river depletion from pumping, which translates to an underestimation  
199 of drawdown to aquifer water levels. The situation would affect accounting of ASR recharge credit.  
200 We inspected flow on the Little Arkansas River during the drought of 2011 and 2012 and found flow  
201 was less than 1 cfs about 30 percent of the time; on the Arkansas River, flow was less than 10 cfs  
202 over 20 percent of the time. Those quantities of flow can be depleted by the City pumping 40,000  
203 AF/y (i.e. Figure 2 shows pumping 40,000 AF/y causes 10 cfs of river depletion in the first year of  
204 pumping and 20 cfs by the second year). Accordingly, we recommend calibrating the model with a  
205 river boundary condition that accounts for routed streamflow to improve conditions represented in  
206 adjacent rivers.

207

### 208 **Drawdown to Aquifer Water Levels and Well Impacts**

209

210 Scenarios A, B and C described above provide a basis for an examination of an example City  
211 pumping condition that draws down water levels to the proposed minimum index level (i.e.  
212 diverting 40,000 AF/y and ASR recharge credit water). **Figure 6** shows drawdown from each of the  
213 scenarios at the eighth year of the drought.<sup>4</sup> The drawdown illustrates an example of potential  
214 water-level impacts from City pumping if the Proposal is approved. Information on local wells can  
215 be compared to the drawdown on **Figure 6** to assess potential impacts to well water columns. The  
216 total drawdown to the proposed minimum index level is the sum of drawdown from Scenarios A, B  
217 and C.

218

---

<sup>4</sup> On Figure 6, Scenario B shows generally less than 1 foot of drawdown in most of the BSA.

219 For an assessment of potential impacts to local wells, we accessed well construction  
220 information from the Water Well Completion Records (WWC5) database available from the Kansas  
221 Geological Survey (KGS).<sup>5</sup> We supplemented the WWC5 well data with additional well  
222 information<sup>6</sup> provided by the Intervenor (Ecomm, 2019). We then mapped the wells in the area of  
223 the drawdown resulting from Scenarios A, B and C and compared total drawdown to well water  
224 columns (less 10 feet)<sup>7</sup> to assess whether the wells could be impacted from lowering water levels to  
225 the proposed minimum index level. **Figure 7** shows 35 wells with water columns less than the total  
226 drawdown from Scenarios A, B and C indicating potential for some wells in the BSA to lose  
227 capacity to produce water as a result of City pumping that could occur if City groundwater  
228 withdrawals cause drawdown to the elevation of the proposed minimum index level. Out of the 35  
229 wells, 29 are impacted from the City pumping 40,000 AF/y and 6 are impacted from pumping ASR  
230 recharge credit down to the minimum index level. The aquifer drawdown assessment herein  
231 represents an example City pumping scheme that causes drawdown to the proposed minimum index  
232 level. Other City pumping schemes are possible that can affect the number of impacted wells.  
233 However, the analysis herein clarifies that the overall magnitude of drawdown to the minimum  
234 index level, caused by City well diversions, exceeds the water column expected to be needed by  
235 some wells in the area.

236

237 **Figure 7** shows the locations of City wells as red circles. The red circles represent the  
238 location of the well and a 660-foot buffer around the well location. We note that out of the wells  
239 potentially impacted, many of them are located at distances greater than 660 feet from the City  
240 wells. This indicates that the minimum well spacing regulation (K.A.R. 5-22-2) for domestic wells  
241 (660 feet), from other wells in a subject application, is not sufficient to provide protection from

---

<sup>5</sup> Since 1975, drilling companies have been mandated by state legislation to provide well information that typically includes well depth and a static depth to water (<http://www.kgs.ku.edu/HighPlains/data/>, data accessed Jan 18, 2019). We quantified well water columns from reported depth to water and well depths in WWC5. The available data is not expected to include all area wells since wells are anticipated to have been drilled prior to 1975.

<sup>6</sup> The information provided for most of the Intervenor wells included a water rights number. For those wells, we cross-referenced water rights numbers with the Water Information Management and Analysis System (WIMAS) to determine well location. For wells that did not have a water rights number, we mapped the wells to the nearest section of the Public Land Survey. Most of the Intervenor wells did not have depth to water. To estimate well water columns, we cross-reference the well locations with a year 2016 digital water-level surface adapted from the USGS (<https://www.sciencebase.gov/catalog/item/5824e0b9e4b0c05b678c45dd>).

<sup>7</sup> We subtracted 10 feet from the well water columns to allow for pump submergence while the well is operating with a pumping water level. This is a general estimate that could be refined in a case-by-case setting if specific area wells are examined for impacts from groundwater pumping.

242 excessive drawdown, suggesting a case-by-case assessment is needed to consider impacts from City  
243 well diversions.

244

## 245 **Water Quality Change**

246

247 The USGS evaluated chloride transport in the Equus Beds aquifer in a preliminary study  
248 (Klager and others, 2014) that is based on the USGS groundwater flow model used for the analysis  
249 described herein. The USGS analysis examines pumping scenarios (and one recharge scenario)  
250 involving variations in regional and municipal pumping to develop an understanding of chloride  
251 displacement that may occur. A summary of selected USGS results is shown on **Figure 8**, which is  
252 adapted from Figure 27 of Klager and others (2014). **Figure 8** shows the greatest potential for  
253 chloride migration is located generally north of the Arkansas River along the southern portion of the  
254 BSA. Similar to the USGS technical approach, we examined chloride displacement based on the  
255 Proposal and found potential for hundreds of feet of displacement of water with chloride resulting  
256 from lowering water levels to the proposed minimum index level; the displacement is generally in  
257 the same location (southern portion of the BSA) as that of the USGS analysis. However, the USGS  
258 notes that modeled chloride in this area moved northeast at a higher rate than is observed in the field  
259 data (Klager and others, 2014, p. 72).<sup>8</sup> Accordingly, the modeled displacement of chloride in this  
260 area is overestimated. The USGS also reports their analysis results indicate potential for the Burrton  
261 plume to continue migrating toward the City wellfield (Klager and others, 2014, p. 72). If the City  
262 diverts groundwater resulting in lowering water levels to the proposed minimum index level, there is  
263 increased potential to induce migration of chloride from the areas of Burrton and the Arkansas  
264 River.

265

266 In the USGS study, the groundwater flow model was not altered to calibrate the solute-  
267 transport model to observed chloride concentration data (Klager and others, 2014, p. 71). The  
268 USGS indicates achieving better performance of chloride transport may require changes to the  
269 groundwater model and that future model updates will allow opportunities for that type of  
270 calibration. We agree with that assessment. In the Proposal, the City does not describe potential  
271 water quality changes associated with lowering the minimum index level. We are not familiar with  
272 the level of detail in which the City has evaluated potential water quality impacts in the context of  
273 the Proposal. Further development of the chloride displacement analysis along the line described by

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<sup>8</sup> In the shallow part of the aquifer, the model simulated chloride movement at a rate about 2x that of observed data; in the deep aquifer, the modeled rate of movement was about 4x that of observed.

274 the USGS is expected to enhance the model for use in assessing potential water quality impacts. We  
275 recommend proceeding along that line in attempt to identify potential issues that may arise if  
276 drought conditions prompt the City to divert groundwater that causes drawdown to the proposed  
277 minimum index level.

278

## 279 **PROPOSED ASR ACCOUNTING METHODOLOGY**

280

281 Section 4.0 of the Proposal describes an accounting method for ASR credits. The method  
282 uses a response-function type of approach that considers a 5 percent initial loss from the BSA the  
283 first year and a 3 percent loss in subsequent years. BMcD shows that the proposed recharge  
284 accounting mirrors the current accounting approach, but with a deviation that occurs when water  
285 levels increase in the BSA.

286

287 BMcD indicates the calculation of tracking recharge credits across the BSA is a very detailed  
288 procedure requiring a substantial amount of data preparation and processing and that there is shared  
289 interest (DWR, GMD2 and the City) in developing a simplified accounting process. Accordingly,  
290 there is utility in simplifying the accounting process with a response-function type of approach.  
291 However, the USGS model accounts for the physical structure of the aquifer system and the  
292 associated change in aquifer system/river response associated with changes in water levels in the  
293 BSA. If using the USGS model is definitively too burdensome, we recommend developing a  
294 response function that accounts for both low and high water levels in attempt to improve the  
295 performance of the simplified accounting method over varying aquifer conditions. Technical  
296 coordination with BMcD would provide insight to the basis behind the proposed simplification  
297 approach and whether development of an alternative response function can provide an improved  
298 simplification technique.

299

## 300 **SUMMARY AND CONCLUSIONS**

301

- 302 1. The City of Wichita currently has a permit for ASR water operations that is conditioned to  
303 allow recovery of ASR recharge credit if water levels are above a specified minimum index  
304 level. The City is proposing to lower the minimum index level to allow for diverting  
305 additional ASR recharge credit that may be needed in the event of a drought.
- 306 2. On behalf of the City, BMcD developed a 1% drought simulation as a basis for the ASR  
307 Permit Modification Proposal. The analysis engine for the simulation is the USGS model of

308 the Equus Beds aquifer (Kelly and others, 2013). The model the City provided to GMD2  
309 differs from the USGS model in that ratio of horizontal to vertical hydraulic conductivity is  
310 modified. We found the modification does not affect the model analysis enough to affect  
311 our overall conclusions described herein. However, unless there is a reason to deviate from  
312 the original USGS model concept, we recommend the City accounting of recharge credit  
313 and the analysis in the Proposal be updated accordingly to confirm that potentially  
314 significant factors do not arise.

315 3. The BMcD analysis of the 1% drought simulation presents hydrologic results in terms of  
316 general water-level elevations and percent of saturated thickness in the Basin Storage Area  
317 (BSA). The model analysis illustrates that pumping proposed during the 1% drought results  
318 in lowering about half of the water levels in the index cells of the BSA below the current  
319 minimum index level. The proposed minimum index level is lower than that derived from  
320 the model simulation. However, BMcD does not present an analysis quantifying hydrologic  
321 effects from pumping that could cause drawdown to that proposed minimum index level.

322 4. We present an analysis of an example scenario in which the City pumps groundwater,  
323 consistent with a goal to utilize 40,000 AF/y from its wellfield prior to use of ASR recharge  
324 credits. The scenario represents diversion of groundwater causing drawdown to the  
325 proposed minimum index level to characterize associated hydrologic effects. We also  
326 illustrate that the USGS model utility for simulation of wells includes capability for  
327 separating the hydrologic effects of City pumping non-credit water from ASR recharge credit  
328 water with consideration of the current and proposed minimum index levels. The  
329 assessment provides insight to hydrologic effects in the context of the new pumping that  
330 could occur if the minimum index levels are lowered.

331 5. The proximity of the City wells to the Little Arkansas and Arkansas rivers, and the aquifer  
332 properties, results in a high degree of connection between groundwater in the BSA and the  
333 rivers. In the first year of City pumping, approximately 20 percent of the pumping amount is  
334 depleted from the river system. If the City diverts ASR recharge credit water causing  
335 drawdown to the proposed minimum index level, a new depletion is anticipated to occur at  
336 the Valley Center MDS gage. Given that the Proposal is based on pumping during drought  
337 conditions, the impact is consistent with a time when MDS gage flows are a concern. The  
338 MDS flow at the Valley Center gage is 20 cfs every month of the year.

339 6. We recommend calibrating the USGS model with a representation of rivers that accounts for  
340 total streamflow. During drought conditions, flow on the Little Arkansas and Arkansas  
341 rivers has lowered to quantities compatible with estimated stream depletion from City

342 groundwater pumping. In that setting, there is potential for overestimating stream depletion,  
343 which translates to an underestimation of aquifer storage depletion. Refining the technique  
344 of modeling the rivers would improve representation of local hydrologic conditions and may  
345 translate to an improved account of ASR recharge credit.

346 7. We examined aquifer water-level response in the BSA from an example of City groundwater  
347 pumping that causes drawdown to the level of the proposed minimum index level. The  
348 drawdown is caused by the City pumping 40,000 AF/y in combination with diverting ASR  
349 recharge credit. The total drawdown is up to approximately 30 feet. We compared the  
350 drawdown from the scenario to information on local well water columns. The result  
351 indicates that up to 35 wells are identified with potential to lose capacity to produce water  
352 from the total drawdown. Out of the 35 wells, 29 are impacted from the City pumping  
353 40,000 AF/y and 6 are identified to be impacted from the City diverting ASR recharge  
354 credits down to the proposed minimum index level. This observation indicates that some  
355 wells in the area can be reasonably anticipated to require a remedy associated with lowering  
356 water levels to the proposed minimum index level. Information on local well water columns  
357 is from the WWC5 database that includes records beginning in 1975. Accordingly, we  
358 anticipate the drawdown assessment does not include all of the local area wells. Some of the  
359 wells (domestic) are located greater than 660 feet from City wells indicating the minimum  
360 well spacing regulation (K.A.R 5-22-2) is not sufficient to provide protection from excessive  
361 drawdown caused by City pumping. This observation suggests a case-by-case assessment is  
362 needed to consider impacts from City well diversions.

363 8. Preliminary USGS study of chloride transport indicates potential for migration from the  
364 Burrton area and generally north of the Arkansas River along the southern portion of the  
365 BSA. The USGS notes that modeled chloride (along the southern portion of the BSA)  
366 moved northeast at a higher rate than is observed in the field data. If the City diverts  
367 groundwater resulting in lowering water levels to the proposed minimum index level, there is  
368 increased potential to induce migration of chloride from the areas of Burrton and the  
369 Arkansas River toward other wells in the area. The USGS chloride transport analysis was  
370 based on the existing groundwater flow model without alterations to improve performance  
371 of the solute transport model. It would be prudent to proceed with further development of  
372 the chloride displacement analysis in attempt to identify potential issues that may arise if  
373 drought conditions prompt the City to divert groundwater that causes drawdown to the  
374 proposed minimum index level.

375 9. The calculation of tracking recharge credits is reported to be a detailed process. The  
376 Proposal describes a response-function type of approach to simplify the accounting method.  
377 The simplified approach deviates from the current approach under conditions of high water  
378 levels. The USGS model provides the best approach for accounting. If a simplified  
379 approach is necessary, we recommend development of a response function that accounts for  
380 both low and high water levels in attempt to improve the simplified accounting method over  
381 varying aquifer conditions.

382

### 383 **QUALIFICATIONS AND EXPERIENCE**

384

385 I am a Certified Professional Hydrologist (08-HGW-1817) with the American Institute of  
386 Hydrology. I am President of Balleau Groundwater, Inc. and have over 20 years of experience in  
387 major aspects of hydrology and hydrogeology with emphasis on analysis of hydrologic processes  
388 involving the interaction of groundwater and surface-water. Development of field-testing programs,  
389 assessment of wellfield performance and yield, water-resource planning and management, arid zone  
390 hydrology, artificial recharge, mine dewatering and water rights litigation support have also been  
391 major activities.

392

393 I have developed, adapted or worked with more than 100 hydrogeologic models. My  
394 experience includes analysis of the local and regional water budgets for both natural hydrologic  
395 conditions and changes induced to the natural system from development of surface water and  
396 groundwater. I have evaluated recharge and recovery of groundwater credit water in southwestern  
397 New Mexico and peer reviewed analyses of artificial recharge in southern California. Over the past  
398 decade I have analyzed hydrologic effects in the Rattlesnake Creek Basin in the area of Stafford  
399 County, Kansas and I am one of the authors of the model currently used by KDA-DWR for analysis  
400 of hydrologic effects in the area of Groundwater Management District No 5. I have advised cities  
401 and peer reviewed hydrogeologic analyses for municipal water districts regarding water resources in  
402 settings that involve groundwater pumping, artificial aquifer recharge, aquifer recharge from  
403 flooding and remediation of groundwater contamination. I have also advised industrial water users,  
404 irrigation and conservancy districts, state and federal agencies, Indian tribes, water associations and  
405 private water users with matters involving source water availability. I have presented at conferences  
406 involving groundwater hydrology and I have been invited to publish in a Theme Issue of the peer-  
407 reviewed journal *Groundwater* on research related to analysis of groundwater flow.

408

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I attest to the contents and substance of the above report.



Dave M. Romero, P.H.



Date: Feb 18, 2019

418 **REFERENCES**

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426

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429

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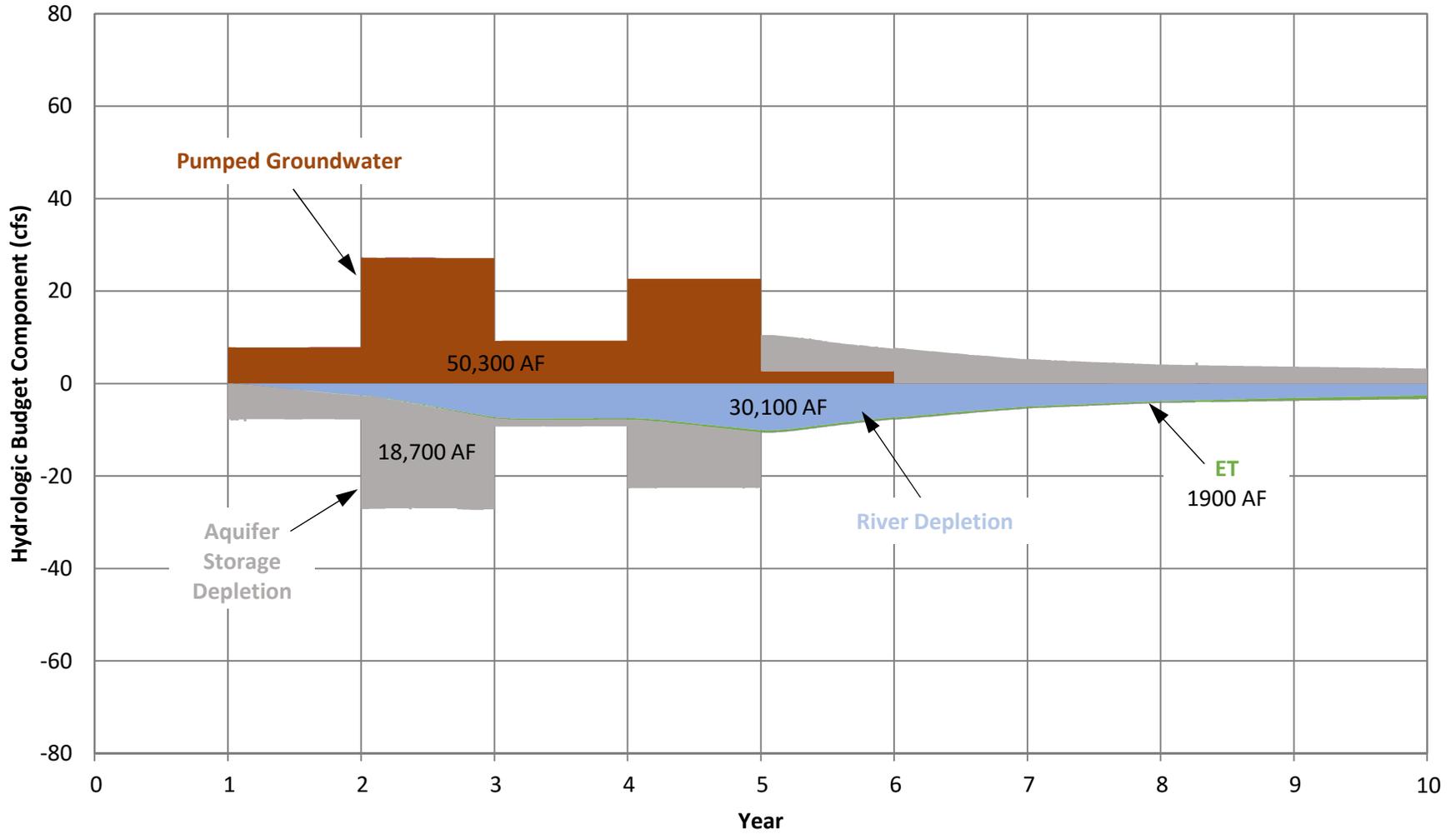
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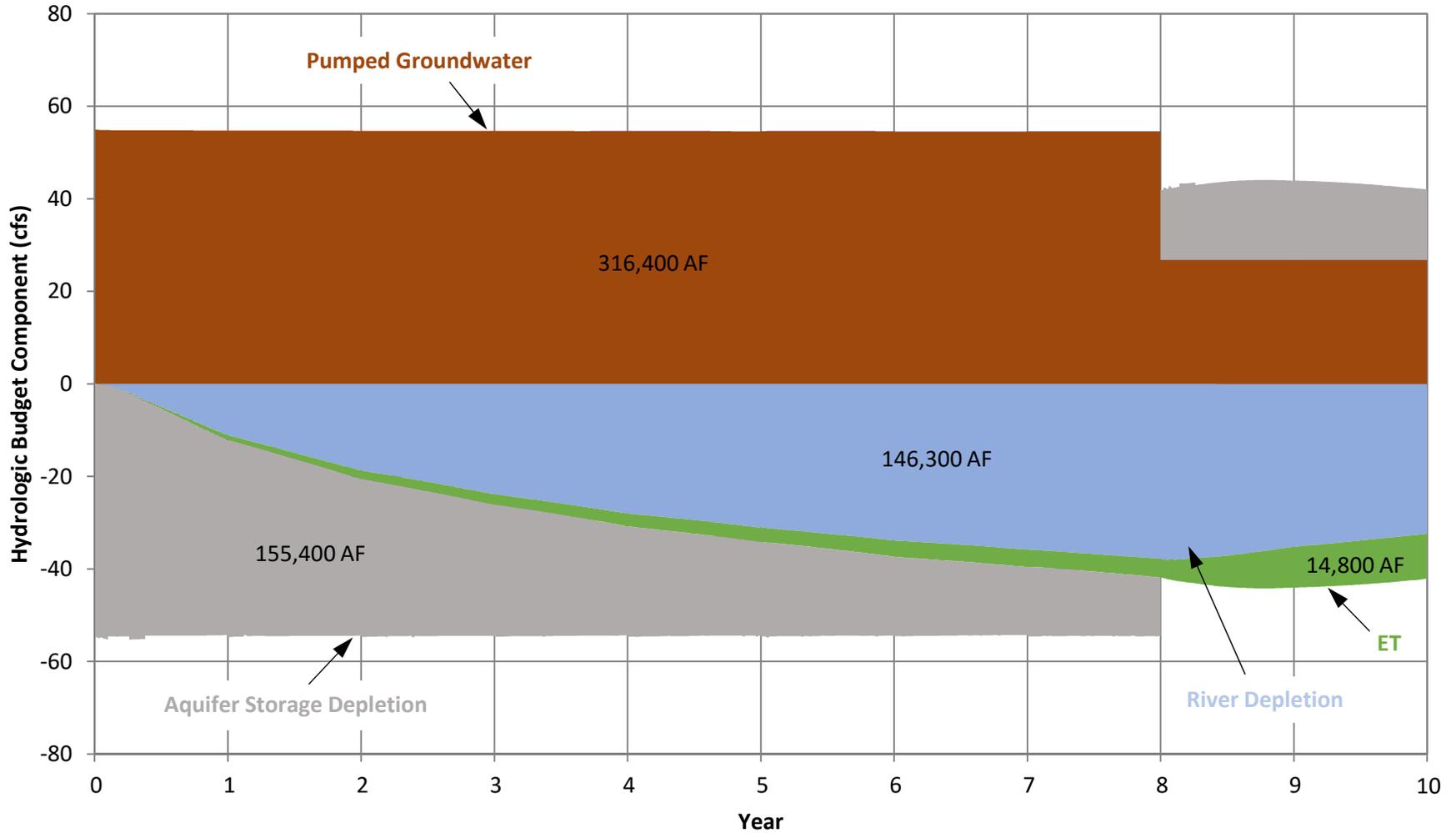
451 [Macey] Letter dated Sep 28, 2018 from Scott A. Macey, Water Resources Engineer with the City of  
452 Wichita Department of Public Works & Utilities to Mr. Tim Boese, District Manager of the  
453 Equus Beds Groundwater Management No.2.

**FIGURE 1**  
**HYDROLOGIC SYSTEM RESPONSE TO CITY PUMPING ASR RECHARGE CREDIT**



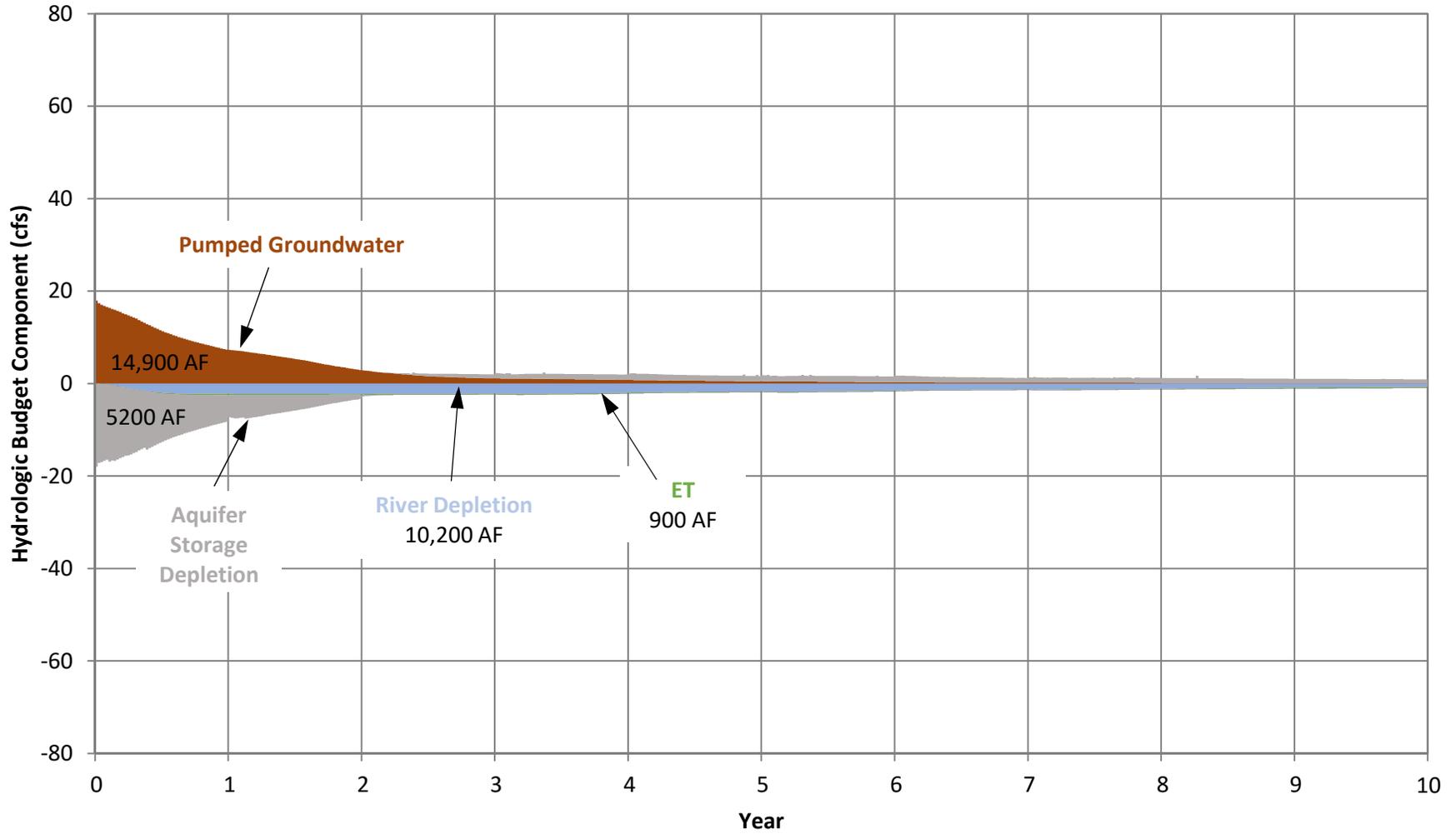
Note: Budget volumes are over 8-year period

**FIGURE 2**  
**HYDROLOGIC SYSTEM RESPONSE TO CITY PUMPING 40,000 AFY (SCENARIO A)**



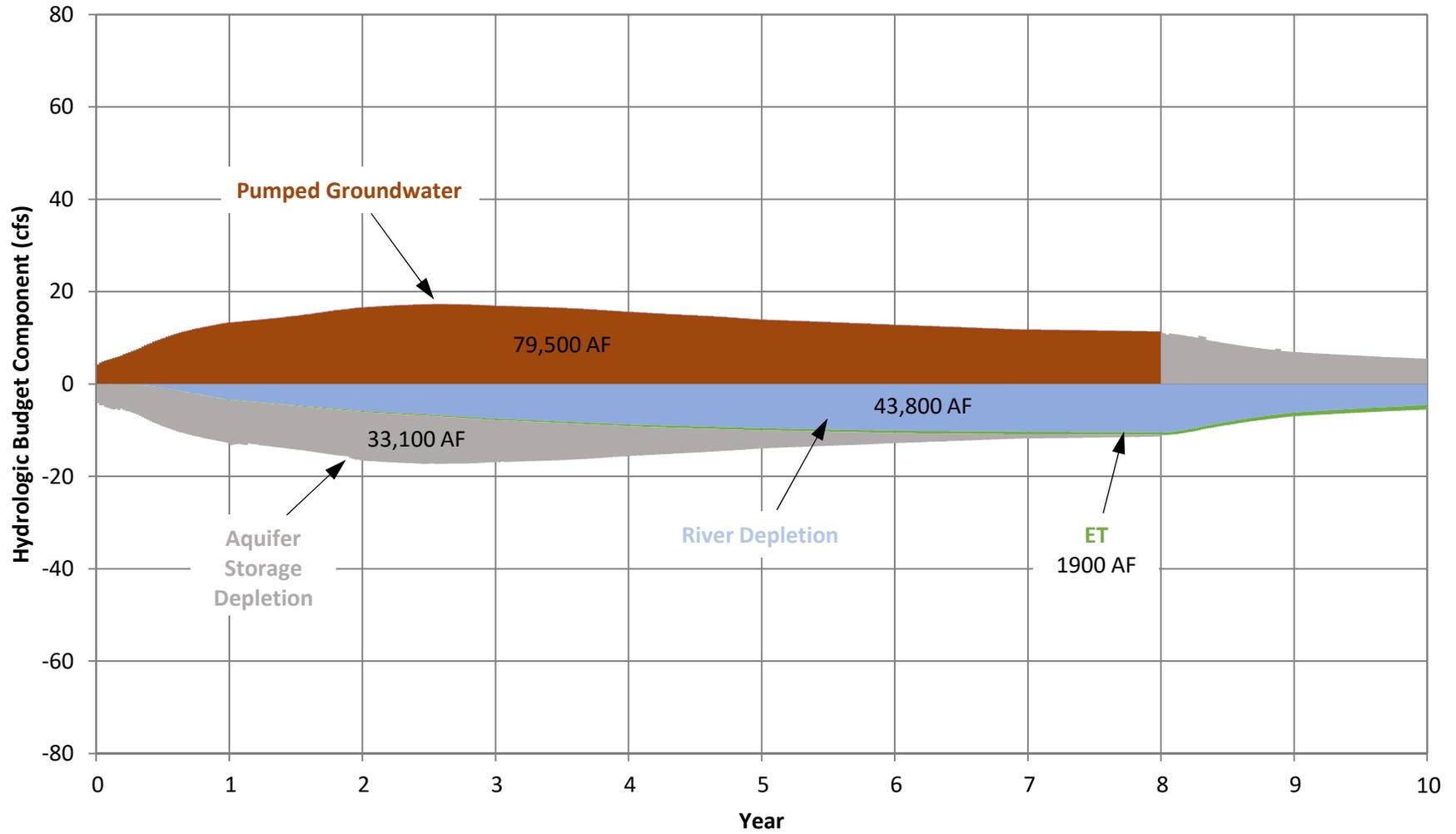
Note: Budget volumes are over 8-year period

**FIGURE 3**  
**HYDROLOGIC SYSTEM RESPONSE TO CITY PUMPING PERMITTED ASR RECHARGE CREDIT (SCENARIO B)**



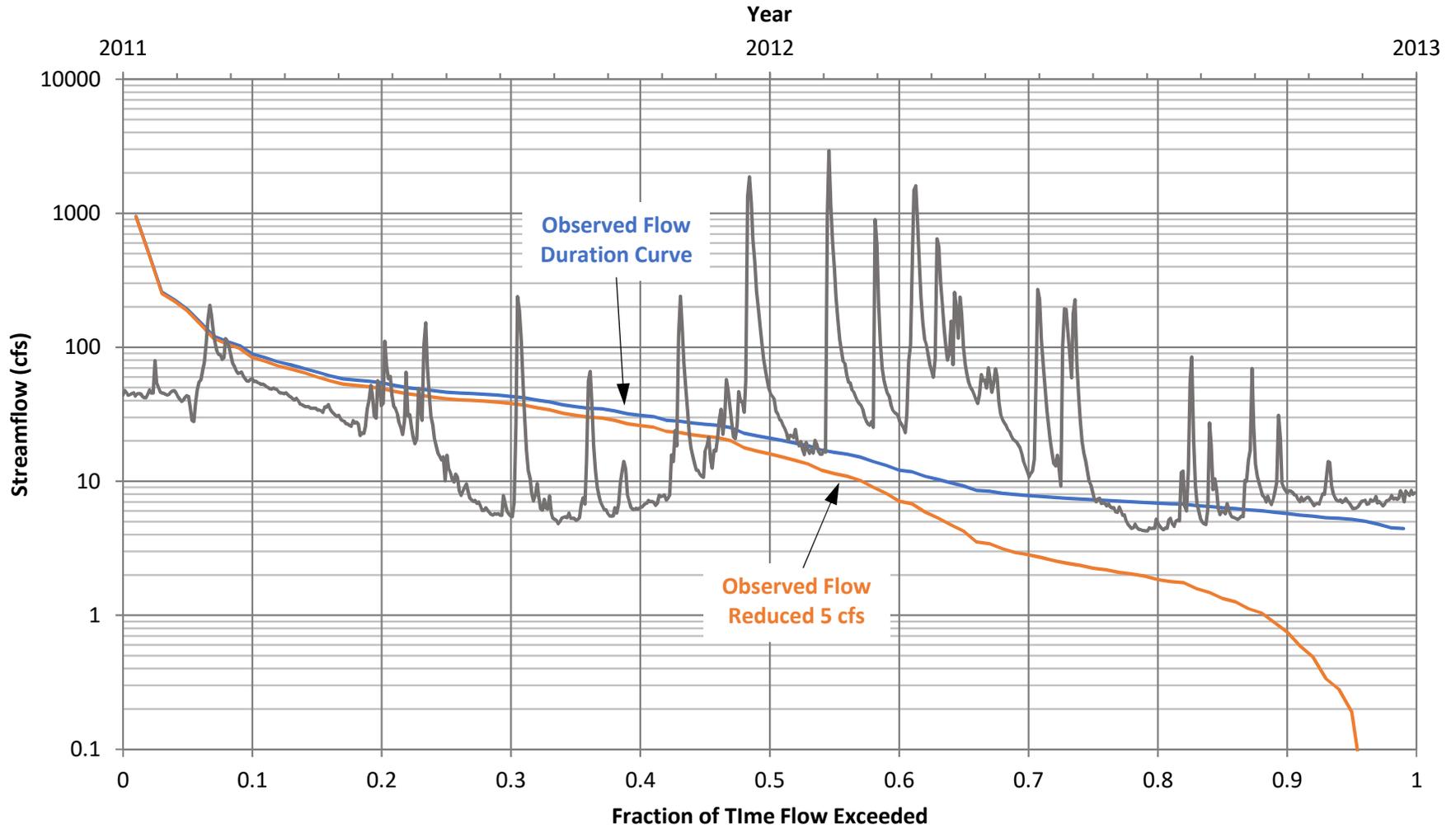
Note: Budget volumes are over 8-year period

**FIGURE 4**  
**HYDROLOGIC SYSTEM RESPONSE TO CITY PUMPING PROPOSED ASR RECHARGE CREDIT (SCENARIO C)**

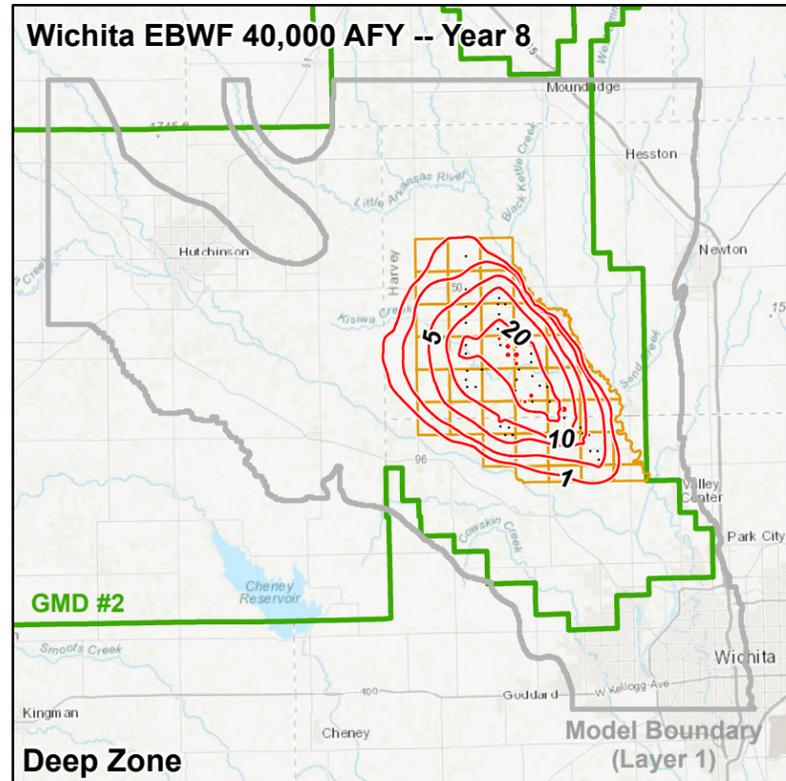


Note: Budget volumes are over 8-year period

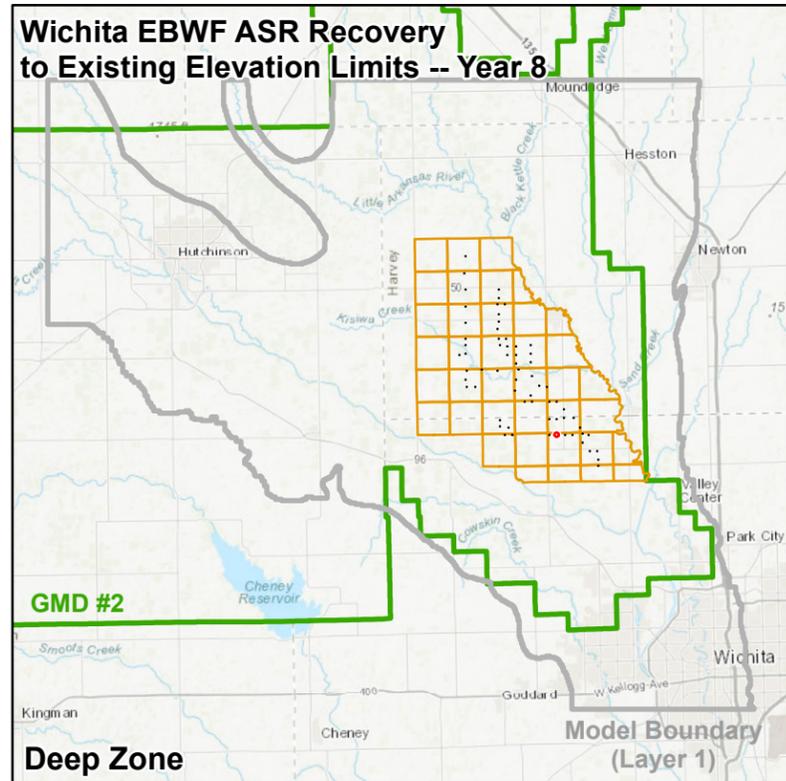
**FIGURE 5**  
**FLOW ON LITTLE ARKANSAS RIVER AT VALLEY CENTER (USGS 07144200)**



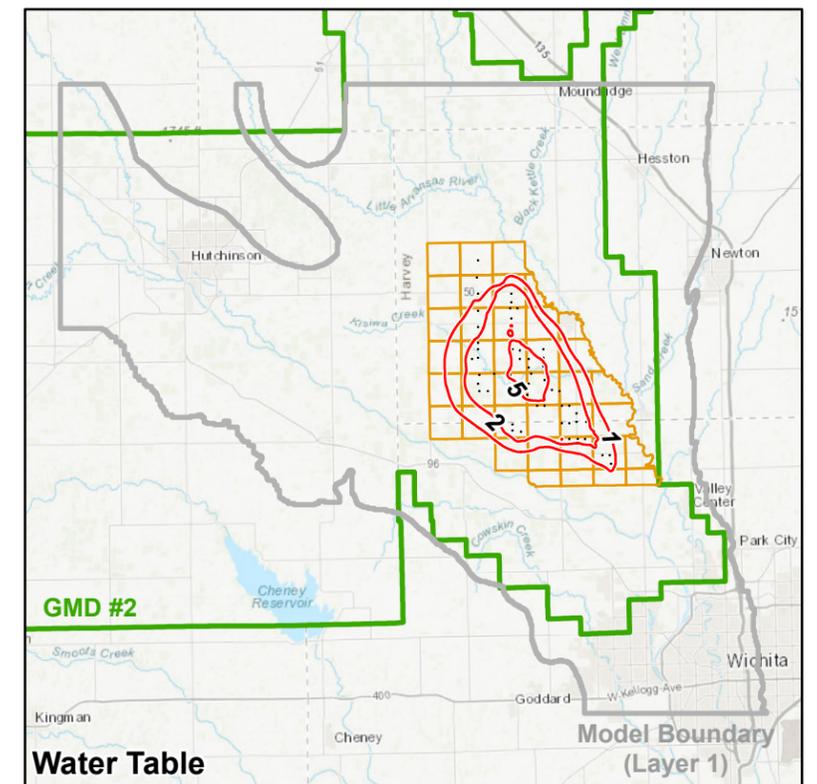
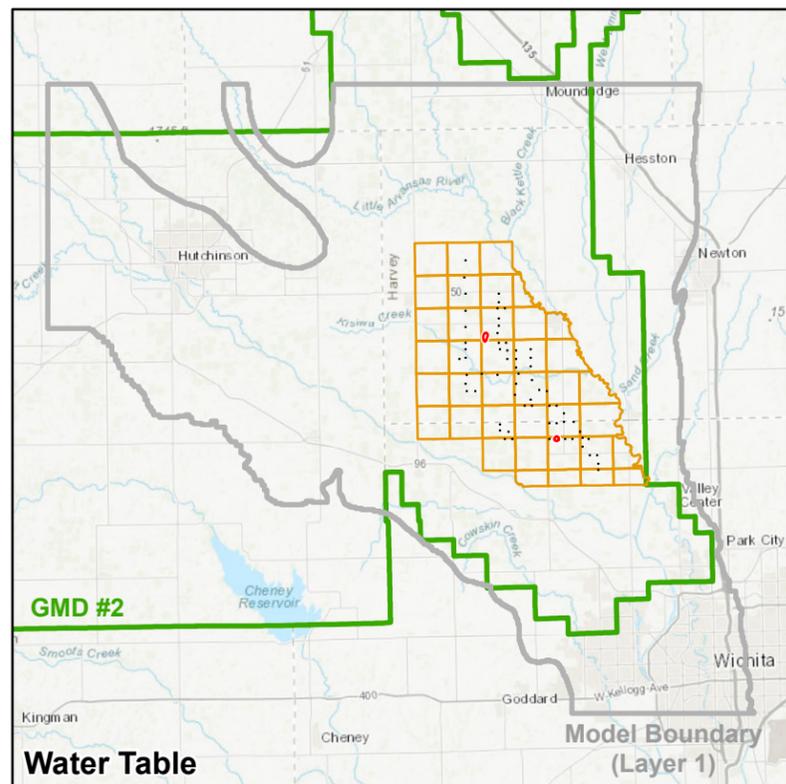
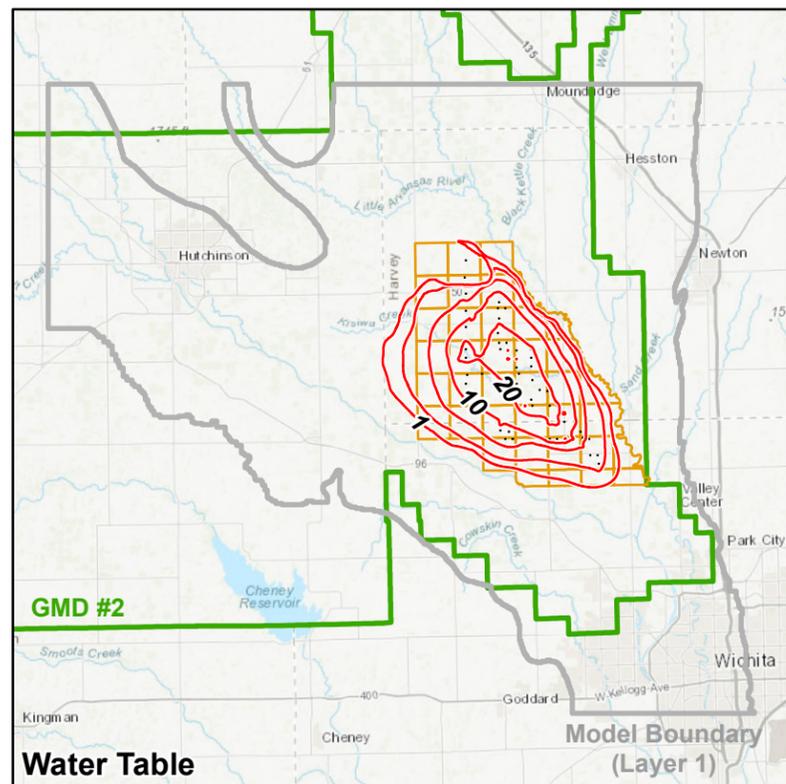
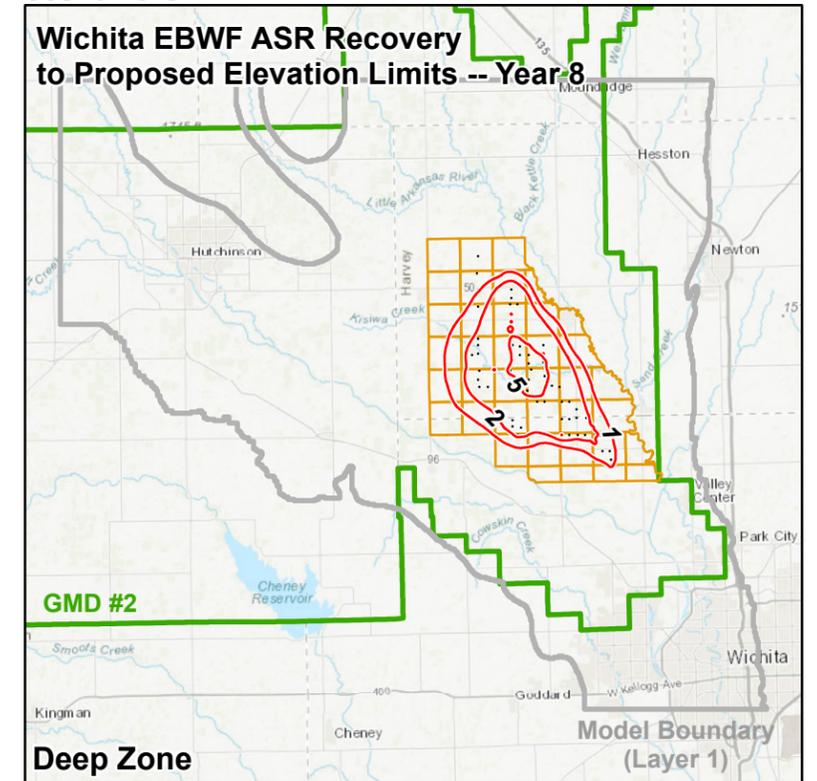
**Scenario A**



**Scenario B**



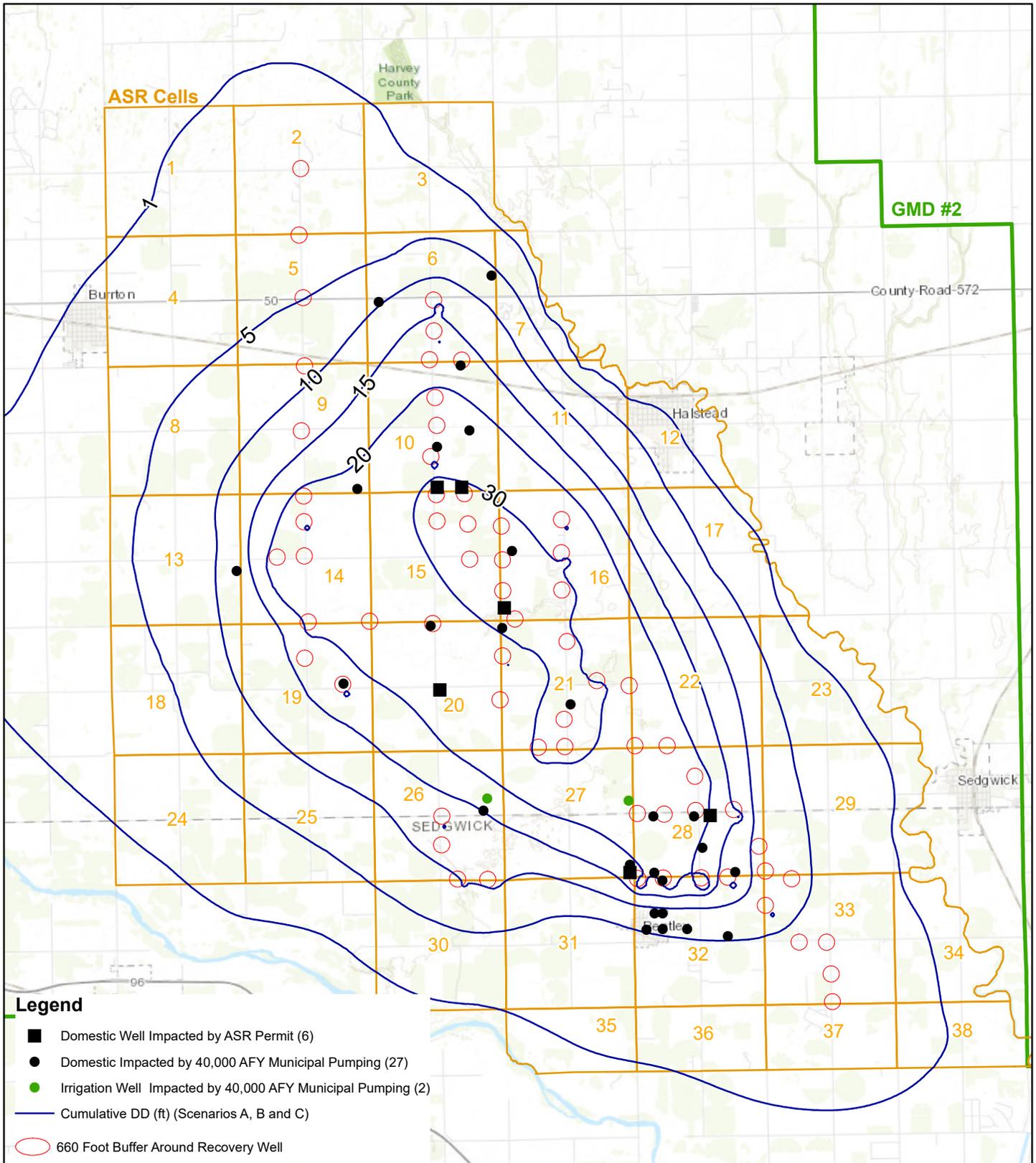
**Scenario C**



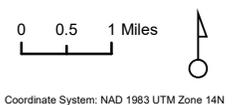
Contour interval = 1, 2, 5, 10 and 20 ft.

**WATER-LEVEL DRAWDOWN FROM SCENARIOS A, B and C**  
**FIGURE 6**



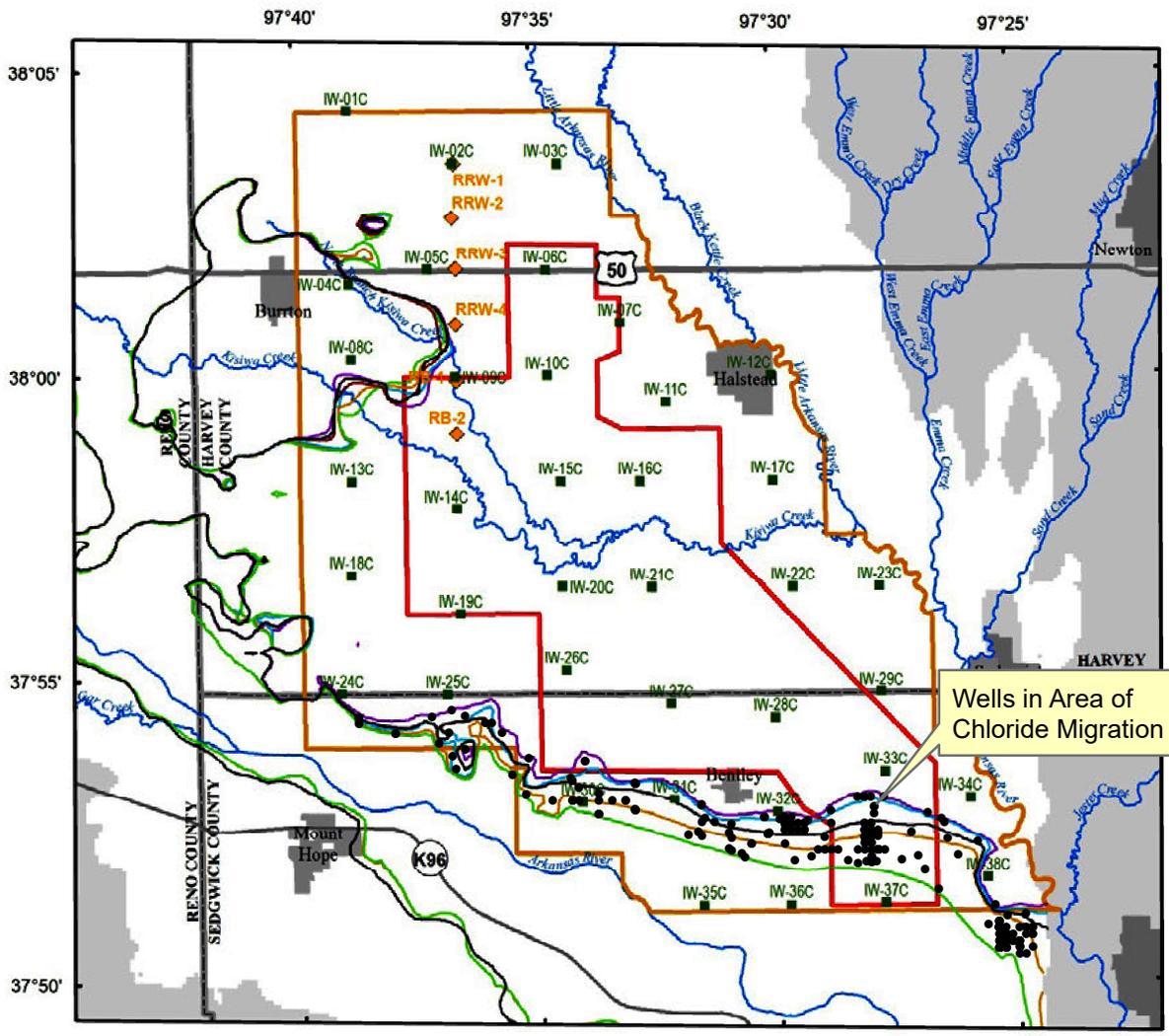


**WELLS PROJECTED TO LOSE CAPACITY  
TO PRODUCE WATER FROM  
WATER-LEVEL DRAWDOWN TO  
PROPOSED MINIMUM INDEX LEVEL**

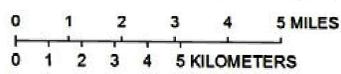


**FIGURE 7**

**Data Sources**  
 Base VIA ESRI online.  
 Recovery wells from GMD #2 data.  
 Well locations from WWC5 data, accessed January 18, 2019.



Base modified from U.S. Geological Survey  
 1:100,000-scaled digital data, 2005  
 Universal Transverse Mercator projection  
 Zone 14  
 Horizontal coordinate information is referenced to the  
 North American Datum of 1983 (NAD83)



**EXPLANATION**

- Inactive areas in model layer 2
- Central Wichita well field
- Basin storage area (BSA)
- Simulated 250-mg/L-chloride fronts for six scenarios**
- Existing pumping (baseline)
- No pumping
- Double Wichita municipal pumping and existing irrigation pumping
- Existing Wichita municipal pumping and no irrigation pumping
- Double Wichita municipal pumping and no irrigation pumping
- Increased Phase 1 artificial recharge (same as baseline in southern part of map)
- Index monitoring well
- Phase I recharge site

**ADAPTED USGS FIGURE 27  
 KLAGER AND OTHERS (2014)  
 FIGURE 8**

